

Plain concrete footing

Unreinforced strip or pad footing · EN 1992-1-1 §12.9.3 · UK NA / PN-EN

Reference	NAE-RC-032 · version 1.0
Standards	BS EN 1992-1-1:2004 with UK NA · PN-EN 1992-1-1:2008 with Polish NA (NDP sources listed on the NDP sheet) · EN 1990 load combination by the user
National Annex	Selectable: UK NA to BS EN 1992-1-1 or Polish NA to PN-EN 1992-1-1
Issued	29 October 2026

What it does

Depth of a plain (unreinforced) concrete strip footing under a wall or pad footing under a column, carrying an axial load: the projection beyond the face against the depth by (12.13), the simplified rule $h_F / a \geq 2$, and the gross bearing pressure against an allowable value.

How to use

1. Open the Calculation sheet and select the National Annex.
2. Fill in the shaded input cells. Everything else is calculated.
3. Read the Results block; every step below it quotes its clause. Print or save as PDF for your calculation package.

Scope and limitations

Axially loaded strip or pad footing of plain concrete to 12.9.3, the wall or column central (no moment, no eccentricity), static loads: dynamic effects (rotating machinery, traffic) excluded as 12.1(2). (12.13): $0.85 h_F / a \geq \sqrt{3 \sigma_{gd} / f_{ctd,pl}}$, with a the larger projection from the face of the wall or column, h_F the depth and $f_{ctd,pl} = \alpha_{ct,pl} f_{ctk,0.05} / \gamma_c$ (3.16, 12.3.1), $\alpha_{ct,pl} = 0.8$ in both annexes (UK NA Amd 1 keeps 0.8 for $\alpha_{ct,pl}$, its $\alpha_{cc,pl} = 0.6$ is not used here; PL NA the recommended value). σ_{gd} is taken as the gross design ground pressure, the footing and backfill weights included (conservative: they cause no bending). The simplified rule $h_F / a \geq 2$ (12.9.3(1)) is shown for information; the check is (12.13). Bearing is checked with characteristic loads against an allowable gross pressure from the geotechnical engineer; EN 1997-1 bearing resistance, sliding and settlement are not covered. Mass concrete footings to old practice (load spread at 45°, $h_F \geq a$) are not the Eurocode rule and are not used. Concrete classes up to C50/60. The second-generation EN 1992-1-1:2023 is not yet implemented.

Further reading and independent cross-check

- The Concrete Centre, How to design concrete structures using Eurocode 2: Foundations (plain concrete footings).
 - The Concrete Centre, Concise Eurocode 2, 12 (plain concrete).
 - The Concrete Centre, RC Spreadsheets v4 (licensed product) can be used as an independent cross-check.
- NA Engineering is not affiliated with The Concrete Centre or CIRIA; their publications are referenced for information only.

About NA Engineering

NA Engineering is a structural engineering consultancy delivering structural design, reinforced concrete modelling and detailing, and steel detailing for residential, commercial, infrastructure, energy and refurbishment projects. We share tools like this one because good engineering should be transparent: every formula here is visible and referenced. If you need a design, a check or a full detailing package, talk to us.

Website	naengineering.uk
Email	adrian@naengineering.uk
Phone	+48 505 844 422
LinkedIn	linkedin.com/company/naengineering

Disclaimer

This spreadsheet is provided free of charge, as is, for use by competent engineers. It does not replace engineering judgement. The user is responsible for the input, for checking that the method suits the element being designed and for verifying the results. NA Engineering accepts no liability for any loss arising from its use. Sheets are password-protected and the workbook structure is locked so that the formulas and the NA Engineering details cannot be changed; only the shaded input cells can be edited.

Revision history

v1.0 29 Oct 2026 · First issue

Project
Element
Date

Job no.
Made by
Checked

Inputs · geometry

Shaded cells only

	National Annex		UK NA (BS EN)	
Table 3.1	Concrete strength class		C25/30	
	Footing		Strip (wall; per metre)	
	Footing width (across the wall; pad: along x)	B	900	mm
	Pad: footing length along y (strip: not used)	L	900	mm
	Depth of the footing	h_F	500	mm
	Wall thickness (pad: column side along x)	t	300	mm
	Pad: column side along y	c_y	300	mm

Inputs · loads and ground

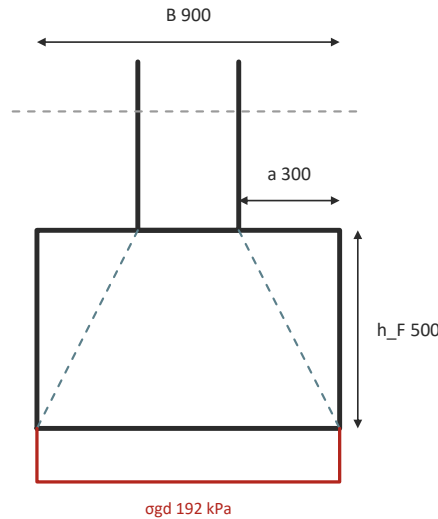
At the top of the footing; strip per metre run

	Load, permanent (kN/m strip, kN pad)	G_k	80.0	kN	
	Load, variable	Q_k	30.0	kN	
EN 1990 A1.2(B)	Partial factor, permanent	γ_G	1.35		
EN 1990 A1.2(B)	Partial factor, variable	γ_Q	1.50		
	Depth of backfill over the footing	D_s	300	mm	
	Unit weight of backfill	γ_{soil}	18.0	kN/m ³	
	Allowable gross bearing pressure	q_{allow}	150	kPa	From the ground investigation
	Inputs valid and consistent				OK

Results

Depth $\sqrt{(3 \sigma_{gd} / f_{ctd,pl}) / (0.85 h_F / a)}$ (12.13)		0.55		OK
Bearing pressure, characteristic q / q_{allow}	q	0.92		OK
Least depth by (12.13), rounded up	$h_{F,min}$	268	mm	
Simplified rule $h_F / a \geq 2$ (for information)	h_F / a	1.67		not met

Your footing · section across the wider projection: the footing, the wall or column, the projection a and the depth h_F



Calculation · ground pressure

2.4.2.4(1)	Partial factor, concrete	γ_c	1.50	NDP sheet
Table 3.1	Characteristic cylinder strength	f_{ck}	25	MPa
Table 3.1	Mean tensile strength	f_{ctm}	2.56	MPa
	Base area (strip: $B \times 1$ m; pad: $B \times L$)	A	0.900	m ²
	Footing and backfill $25 h_F A + \gamma_{soil} D_s$ (A – wall or column)	W	14.49	kN
	Characteristic gross pressure $(G_k + Q_k + W) / A$	q_k	138.3	kPa
	Against the allowable pressure	q_{allow}	150	kPa
EN 1990 (6.10)	Design gross ground pressure $(\gamma_G (G_k + W) + \gamma_Q Q_k) / A$	σ_{gd}	191.7	kPa

Calculation · depth, 12.9.3

Table 3.1	Characteristic tensile strength (5 % fractile)	$f_{ctk,0.05}$	1.80	MPa
12.3.1	$\alpha_{ct,pl}$ (UK NA Amd 1 and PL NA: 0.8)	$\alpha_{ct,pl}$	0.8	
(3.16), 12.3.1	Design tensile strength $\alpha_{ct,pl} f_{ctk,0.05} / \gamma_c$	$f_{ctd,pl}$	0.958	MPa
12.9.3(1)	Projection beyond the face (larger of the two for a pad)	a	300	mm
(12.13)	Left side $0.85 h_F / a$		1.417	
(12.13)	Right side $\sqrt{(3 \sigma_{gd} / f_{ctd,pl})}$		0.775	
(12.13)	$0.85 h_F / a \geq \sqrt{(3 \sigma_{gd} / f_{ctd,pl})}$		1.417	OK
	Pressure without the footing weight $\sigma_0 = \sigma_{gd} - 0.025 \gamma_G h_F$	σ_0	174.9	kPa
(12.13)	Least depth $(0.85 h / a)^2 = 3 (\sigma_0 + 0.025 \gamma_G h) / (1000 f_{ctd,pl})$, so	$h_{F,min}$	267.9	mm
	Depth to adopt at least (rounded up to the mm)	h_F	268	mm

The least depth includes the weight of that footing. The footing must be cast against undisturbed ground or on blinding, and the concrete class chosen for the exposure (EN 206 / BS 8500).

Prepared with NA Engineering calculation tools · naengineering.uk · adrian@naengineering.uk. Free for use by competent engineers; see the About sheet for scope and disclaimer.

Nationally Determined Parameters

Values used by the Calculation sheet for the selected National Annex. Sources: UK NA to BS EN 1992-1-1:2004, Table NA.1; PN-EN 1992-1-1:2008, Załącznik krajowy NA. Edit only if your project specifies different values.

Parameter	Clause	UK NA	PL NA	Notes
Partial factor, concrete γ_c	2.4.2.4(1)	1.5	1.4	Persistent & transient. UK NA: recommended; PL: NA.2, Table NA.2
Partial factor, reinforcement γ_s	2.4.2.4(1)	1.15	1.15	UK NA: recommended; PL: NA.2, Table NA.2
α_{cc} (compression in flexure / axial)	3.1.6(1)	0.85	1	UK NA: 0.85; PL: recommended (NA.9)
$\alpha_{cc,pl}$ (plain concrete, compression)	12.3.1(1)	0.6	0.8	UK NA Table NA.1: 0.6; PL: recommended (NA.9)
α_{cc} (shear and punching)	3.1.6(1)	1	1	UK NA: 1.0 for other phenomena; PL: recommended (NA.9)
α_{ct} (design tensile strength)	3.1.6(2)	1	1	Recommended value in both annexes
CRd,c numerator (CRd,c = value / γ_c)	6.2.2(1), 6.4.4(1)	0.18	0.18	Recommended value in both annexes
k1 (axial stress term, punching)	6.4.4(1)	0.1	0.1	Recommended value in both annexes
vRd,max factor (vRd,max = factor · v · fcd)	6.4.5(3)	0.5	0.4	UK NA: 0.5 v fcd; PL: recommended value (NA.9), 0.4 v fcd since EN 1992-1-1 AC:2010 (PN-EN 1992-1-1:2008)
kmax (vEd ≤ kmax · vRd,c at u1)	6.4.5	2	1.5	UK NA 6.4.5(3): 2 vRd,c. PL: no limit in PN-EN 1992-1-1:2008; 1.5 (A1:2014 recommendation) applied conse
ϕ_{large} (large bar threshold, mm)	8.8(1)	40	32	UK NA: 40 mm; PL: 32 mm recommended (NA.9)
cot θ upper limit (variable strut)	6.2.3(2)	2.5	2	UK NA: 2.5 (1.25 with axial tension); PL: NA.4, $1 \leq \cot \theta \leq 2.0$
k1 (x/d limit, $\delta \geq k1 + k2 x_u/d$)	5.5(4)	0.4	0.44	UK NA for $f_{yk} \leq 500$ MPa; PL: recommended (NA.9)
k2 (x/d limit, $\epsilon_{cu2} = 0.0035$)	5.5(4)	1	1.25	UK NA: $0.6 + 0.0014/\epsilon_{cu2}$; PL: $1.25(0.6 + 0.0014/\epsilon_{cu2})$
Upper limit on the (310/ σ_s) factor (0 = none)	7.4.2(2)	1.5	0	UK NA Table NA.5 Note 5: (310/ σ_s) or (500/ f_{yk})($A_s,prov/A_s,req$) ≤ 1.5; PL: none
L/d not to exceed 40 K (1 = applies)	7.4.2(2)	1	0	UK NA Table NA.5 Note 6; not in EN text
ϕ_{min} (longitudinal bars in columns, mm)	9.5.2(1)	12	6	UK NA: 12 mm; PL: NA.6, 6 mm
k3 (crack spacing)	7.3.4(3)	3.4	3.4	Recommended value in both annexes
k4 (crack spacing)	7.3.4(3)	0.425	0.425	Recommended value in both annexes