

Crack control by tables

Bar size and spacing limits · EN 1992-1-1 §7.3.3 · UK NA / PN-EN

Reference	NAE-RC-016 · version 1.0
Standards	BS EN 1992-1-1:2004 with UK NA · PN-EN 1992-1-1:2008 with Polish NA (NDP sources listed on the NDP sheet)
National Annex	Selectable: UK NA to BS EN 1992-1-1 or Polish NA to PN-EN 1992-1-1
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What it does

Control of cracking without direct calculation (EN 1992-1-1 7.3.3): the steel stress under the quasi-permanent load, and the largest bar size (Table 7.2N, adjusted by (7.6N) or (7.7N)) and bar spacing (Table 7.3N) for the chosen crack width, with the slab exemption of 7.3.3(1).

How to use

1. Open the Calculation sheet and select the National Annex.
2. Fill in the shaded input cells. Everything else is calculated.
3. Read the Results block; every step below it quotes its clause. Print or save as PDF for your calculation package.

Scope and limitations

Cracking caused mainly by loading: either Table 7.2N or Table 7.3N is enough (7.3.3(2) Note); for restraint cracking only Table 7.2N applies, with σ_s the stress just after cracking (enter it directly). Minimum reinforcement to 7.3.2 must be provided. σ_s under the quasi-permanent combination is estimated as $f_{yd} (G_k + \psi_2 Q_k) / (\gamma_G G_k + \gamma_Q Q_k) \times A_{s,req} / A_{s,prov} / \delta$ (TCC How to), or entered. Linear interpolation between the tabulated stresses; stresses below 160 MPa use the 160 MPa row; above the last value with a limit the table gives none. The bar size adjustment uses $f_{ct,eff} = f_{ctm}$ (or the value entered for early-age cracking), $k_c = 0.4$ and $h_{cr} = 0.5 h$ for bending, $h_{cr} = h$ for tension. Choose w_k from the annex (UK NA Table NA.4; EN Table 7.1N used by PL). Slabs in buildings in bending with $h \leq 200$ mm and 9.3 applied need no specific measures (7.3.3(1)).

Further reading and independent cross-check

- EN 1992-1-1, 7.3.2 and 7.3.3.
- The Concrete Centre, How to design concrete structures using Eurocode 2: Getting started (crack control tables).

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Revision history

v1.0	29 Oct 2026 · First issue
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Project
Element
Date

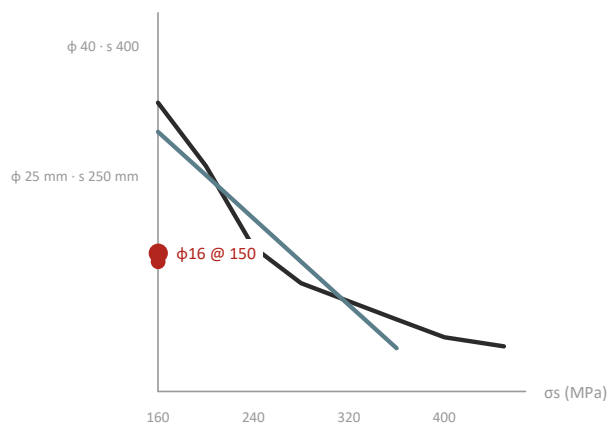
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Inputs · section and bars
Shaded cells only

	National Annex		UK NA (BS EN)	
Table 3.1	Concrete strength class		C32/40	
3.2.2	Reinforcement characteristic yield strength (≤ 500 MPa)	f_{yk}	500	MPa
	Member		Beam or slab in bending	
7.3.1(5)	Design crack width w_k	w_k	0.3	mm
	Overall depth	h	500	mm
	Effective depth (centroid of the outer layer)	d	450	mm
	Bar diameter	φ	16	mm
	Bar spacing	s	150	mm

Inputs · steel stress
Quasi-permanent combination

	Steel stress		Estimate from the loads	
	Steel stress σ_s (if entered)	σ_s	240	MPa
	Permanent load G_k (any consistent unit)	G_k	6.00	
	Variable load Q_k	Q_k	3.00	
EN 1990 A1.1	ψ_2 for the variable load	ψ_2	0.30	
	Partial factors γ_G	γ_G	1.35	
	γ_Q	γ_Q	1.50	
	Reinforcement required at ULS $A_{s,req}$	$A_{s,req}$	900	mm ²
	Reinforcement provided $A_{s,prov}$	$A_{s,prov}$	1340	mm ²
5.5	Moment redistribution δ = redistributed / elastic moment (0.7)	δ	1.00	1.0 = no redistribution
7.3.2(2)	Tensile strength when the cracks form		f _{ctm} (28 days)	
	f _{ct,eff} (if entered: early age, e.g. 3 – 5 days)	$f_{ct,eff}$	2.00	MPa
	Inputs valid and consistent			OK

Tables 7.2N and 7.3N for your w_k · bar size adjusted by (7.6N) / (7.7N) (dark) and spacing (blue, scaled 1:10) · your bars at σ_s (red)

Results

Steel stress, quasi-permanent	σ_s	160	MPa
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Bar size $\phi / \phi_{s,max}$ (Table 7.2N adjusted)	0.48	OK
Bar spacing s / s_{max} (Table 7.3N)	0.50	OK
Crack control (loading: either table is enough)	0.3 mm	OK

Restraint cracking (early thermal, shrinkage): only the bar size check applies, with as the stress just after cracking — enter it directly. The slab exemption needs $h \leq 200$ mm, bending without significant axial tension and the detailing of 9.3.

Calculation

2.4.2.4(1)	Partial factor, reinforcement	γ_s	1.15	NDP sheet
Table 3.1	Characteristic cylinder strength	f_{ck}	32	MPa
Table 3.1	Mean tensile strength	f_{ctm}	3.02	MPa
3.2.7	Design yield strength f_{yk} / γ_s	f_{yd}	434.8	MPa
7.3.3	Quasi-permanent / ULS load ratio $(G_k + \psi_2 Q_k) / (\gamma_G G_k + \gamma_Q Q_k)$		0.548	
7.3.3	Steel stress $f_{yd} \times \text{ratio} \times A_{s,req} / A_{s,prov} / \delta$ (or entered)	σ_s	159.9	MPa
7.3.2(2)	Tensile strength when the cracks form $f_{ct,eff}$	$f_{ct,eff}$	3.02	MPa
(7.6N), (7.7N)	Bar size factor $f_{ct,eff} / 2.9 \times k_c h_{cr} / (2(h - d))$ (tension: $h_{cr} / (8(h - d))$)		1.043	
Table 7.2N	Maximum bar size from the table ϕ_{s*}	ϕ_{s*}	32.0	mm 0: no value in the table
(7.6N), (7.7N)	Adjusted $\phi_{s*} (f_{ct,eff}/2.9) \times k_c h_{cr} / (2(h - d))$ (tension: $h_{cr} / (8(h - d))$)	ϕ_s	33.4	mm
Table 7.3N	Maximum bar spacing s_{max}	s_{max}	300	mm 0: no value in the table
Table 7.2N	Bar size $\phi \leq \phi_s$ (exempt slab $h \leq 200$: not needed)	ϕ	16	mm OK
Table 7.3N	Bar spacing $s \leq s_{max}$ (exempt slab $h \leq 200$: not needed)	s	150	mm OK

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EN 1992-1-1 Table 7.2N (bar size, mm) and 7.3N (spacing, mm)

σ_s (MPa)	ϕ , wk 0.4	ϕ , wk 0.3	ϕ , wk 0.2	s, wk 0.4	s, wk 0.3	s, wk 0.2
160	40	32	25	300	300	200
200	32	25	16	300	250	150
240	20	16	12	250	200	100
280	16	12	8	200	150	50
320	12	10	6	150	100	—
360	10	8	5	100	50	—
400	8	6	4	—	—	—
450	6	5	—	—	—	—

Table notes: $c = 25$ mm; $f_{ct,eff} = 2.9$ MPa; $h_{cr} = 0.5$ h; $(h - d) = 0.1$ h; $k_1 = 0.8$; $k_2 = 0.5$; $k_c = 0.4$; $k = 1.0$; $k_t = 0.4$; $k' = 1.0$.

Nationally Determined Parameters

Values used by the Calculation sheet for the selected National Annex. Sources: UK NA to BS EN 1992-1-1:2004, Table NA.1; PN-EN 1992-1-1:2008, Załącznik krajowy NA. Edit only if your project specifies different values.

Parameter	Clause	UK NA	PL NA	Notes
Partial factor, concrete γ_c	2.4.2.4(1)	1.5	1.4	Persistent & transient. UK NA: recommended; PL: NA.2, Table NA.2
Partial factor, reinforcement γ_s	2.4.2.4(1)	1.15	1.15	UK NA: recommended; PL: NA.2, Table NA.2
α_{cc} (compression in flexure / axial)	3.1.6(1)	0.85	1	UK NA: 0.85; PL: recommended (NA.9)
α_{cc} (shear and punching)	3.1.6(1)	1	1	UK NA: 1.0 for other phenomena; PL: recommended (NA.9)
α_{ct} (design tensile strength)	3.1.6(2)	1	1	Recommended value in both annexes
CRd,c numerator (CRd,c = value / γ_c)	6.2.2(1), 6.4.4(1)	0.18	0.18	Recommended value in both annexes
k1 (axial stress term, punching)	6.4.4(1)	0.1	0.1	Recommended value in both annexes
$v_{Rd,max}$ factor ($v_{Rd,max} = \text{factor} \cdot v \cdot f_{cd}$)	6.4.5(3)	0.5	0.4	UK NA: 0.5 v fcd; PL: recommended value (NA.9), 0.4 v fcd since EN 1992-1-1 AC:2010 (PN-EN 1992-1-1:2008)
kmax ($v_{Ed} \leq k_{max} \cdot v_{Rd,c}$ at u1)	6.4.5	2	1.5	UK NA 6.4.5(3): 2 vRd,c. PL: no limit in PN-EN 1992-1-1:2008; 1.5 (A1:2014 recommendation) applied conse
ϕ_{large} (large bar threshold, mm)	8.8(1)	40	32	UK NA: 40 mm; PL: 32 mm recommended (NA.9)
cot θ upper limit (variable strut)	6.2.3(2)	2.5	2	UK NA: 2.5 (1.25 with axial tension); PL: NA.4, $1 \leq \cot \theta \leq 2.0$
k1 (x/d limit, $\delta \geq k_1 + k_2 x_u/d$)	5.5(4)	0.4	0.44	UK NA for $f_{yk} \leq 500$ MPa; PL: recommended (NA.9)
k2 (x/d limit, $\epsilon_{cu2} = 0.0035$)	5.5(4)	1	1.25	UK NA: $0.6 + 0.0014/\epsilon_{cu2}$; PL: $1.25(0.6 + 0.0014/\epsilon_{cu2})$
Upper limit on the (310/ σ_s) factor (0 = none)	7.4.2(2)	1.5	0	UK NA Table NA.5 Note 5: (310/ σ_s) or (500/ f_{yk})($A_{s,prov}/A_{s,req}$) ≤ 1.5 ; PL: none
L/d not to exceed 40 K (1 = applies)	7.4.2(2)	1	0	UK NA Table NA.5 Note 6; not in EN text
ϕ_{min} (longitudinal bars in columns, mm)	9.5.2(1)	12	6	UK NA: 12 mm; PL: NA.6, 6 mm
k3 (crack spacing)	7.3.4(3)	3.4	3.4	Recommended value in both annexes
k4 (crack spacing)	7.3.4(3)	0.425	0.425	Recommended value in both annexes